



Full-scale

civil infra-structure performance: University of Sheffield experiences 2008-11 (sub text: Structural health monitoring)

James Brownjohn Vibration Engineering Section (VES) Dept of Civil and Structural Engineering, University of Sheffield, United Kingdom

Working with:

Aleksandar Pavic (VES: former EPSRC ARF & GSE review author) Paul Reynolds (VES: EPSRC LF) Keith Worden (Mech. Eng: former EPSRC ARF) Dr Ki-Young Koo



Tivil & Structural Wibration Engineering Section:

Relevant portfolio of grants and experiences

- £ Novel data mining and performance diagnosis systems for structural health monitoring of suspension bridges. EPSRC ~£0.5M
- £ IRIS: Integrated European Industrial Risk Reduction System. EU/FP7 ~0.25M
- £ Dynamic performance of large civil engineering structures: an integrated approach to management, design & assessment. EPSRC Platform, ~0.8M
- £ ~£2.5M other grants now or soon: active control, fellowship, pedestrian loads due to synchronisation (with Birmingham psychologists)
- 200+ dynamic investigations of highway bridges, footbridges, dams, high and low rise buildings, floors, chimneys etc. etc., + dozen structures monitored.

Commercial applications now managed by:

- Experience 1: Humber Bridge
- Experience 2: Tamar Bridge
- Experience 3: Rugeley Chimney
- Experience 5. International Bridge

Experience 1: Humber Bridge Structural Health Monitoring System (optimised sparse array)





Department of Civil & Structural Engineering.



GPS & Wind monitoring System







Accelerometers & Inclinometer







SHM Data management SQL database accessible via MATLAB and Google Apps: powerful data interpretation



*Replication: Robust process of data synchronization even when network link is unstable ** MySQL: a database management system free for non-enterprise uses, owned by Oracle





Internet viewer shows effect of wind on response levels & modal properties for first lateral mode (2011)





Department of Civil & Structural Engineering.

Axial bearing movement:



Exceeds expectation, so redesign modified









What does it mean?

- Bearings look in bad way, but performance is little changed in 20 years. Replacement design influenced (a bit) by our data
- The same bearings probably cause the large swings in lateral mode parameters (0.054Hz is an *extremely low* frequency!)
- Wind has a very strong effect on bridge dynamics

Experience 2 : Tamar Bridge (not so sparse



University of Sheffield Displacements from (R)TS show daily pattern

Follow structural temperature and drive tensions in cables



RTS vertical positioning:

The

- •Linear relationship with extensometer readings & longitudinal data
- •At lower temperatures, cables contract & 'pull' deck up
- •Vertical motion (~210mm) > longitudinal motion (~85mm)

Fundamental vertical vibration mode frequency VS1: primary factor is traffic load, secondary is temperature



Rush-hour traffic jam



Red line is 'residual' effect on dynamics due to thermal effects on structure configuration (e.g. cable tension changes)





RTS-measured displacements for 230 t vehicle









Consultant (AECOM) prediction validated



Saltash Tower longitudinal







How does this inform us?

- Complex static performance (daily dance) mainly driven by temperature
- Dynamic response for vertical modes strongly driven by traffic
- We suspect bearing effects on dynamic performance as at Humber, but harder to pin down
- Consultant and VES FEM validated
- Robotic Total Station data useful for retrofit evaluation and extreme load performance assessment

Experience 3: Rugeley Chimney

New chimney constructed in 2006 for flue-gas desulphurisation (FGD) system. Consultant expected problems due to wind 'interference' effects and commissioned monitoring system and tuned mass damper (TMD)



TMD: largest ever on RC chimney









42 tonne mass (~3% mass ratio) kNs/m damping 450mm travel ->4% damping



Accelerometers and internet viewer



Real time frequency and damping estimation shows max performance of TMD around 4% (29 February 2008)



Shows TMD is working and chimney is in safe displacement range







Why the jump-shift?

Vertical sequence of masonry rings with small 'dust' were filled expansion joint. Data suggest *composite action* of concentric cylinders when gap is closed due to liner expansion. Composite action increases stiffness, hence frequency.



Chimney has now been demolished









How does this inform us?

- Monitoring and real-time modal analysis showed TMD was working well, even after large frequency shift
- Response data showed chimney was in safe working range
- Response data validated the theory of 'enhanced vortex shedding'
- Change in frequencies after taking chimney offline believed to be due to composite action
- Some performance phenomena remain unexplained

a) Imote2 in practice, Helix Bridge Singapore

- Dense mesh of steel sections...
- ... winding around R.C. deck
- Imote2s placed near slab surface
- Antenna close to concrete slab
- Steel structure disrupts radio waves
- High gain antennae and extensions (1m to 2m) proved helpful







Department of Civil & Structural Engineering.



b) NI system checking bearings at Tamar Bridge. Standard solution with exceptional transmission distances



Experience 5: 'International Bridge'









Department of Civil & Structural Engineering.



US 202/NJ 23 Bridge, Wayne, NJ



Knackered expansion joints



Pointless lateral bracing

Fiddly and U/S bearings

- Built 1983
- 'Typical' structure
- Worst rating by NJDOT
- Four span, dual bridge, 7 lane
- Up to 130' by 62'
- Concrete slab on steel stringer
- Visually deficient

Is this hammerhead falling off? So what?



US-202/NJ-23 Bridge: Adopted by FHWA LTBP; International collaboration, driven by Drexel











VES designated role was system identification (we have 'tested' over 200 civil structures worldwide) Vibration test kit: APS400 shaker QA 750 servo accelerometers, (shadowed by Georgia Tech wireless accelerometers)







Modal analysis using NExT/ERA, from random excitation. Mode shapes show transverse mode order, effect of asymmetry and orthotropic properties

Spanwise shape



mode: 1 f=3.07Hz zeta=1.4%

mode: 4 f=5.14Hz zeta=0.66%



mode: 7 f=11.6Hz zeta=1.4%



mode: 2 f=3.67Hz zeta=0.66%



mode: 5 f=5.17Hz zeta=0.59%



mode: 8 f=12Hz zeta=1.8%







half-sine

mode: 6 f=9.42Hz zeta=0.76%



half-sine

full-sine

mode: 9 f=12.2Hz zeta=1.3%







How does this inform us?

- Vibration test is only part of an array of tools: Full program of condition assessment included static load test, GPR, visual inspection (US/Austria/Japan standards) etc. etc..
- Excessive dynamic response needs fixing
- Bridge in in better state than it looks, some defects are cosmetic
- Bearings need replacing
- Multi-span bridge not such a great idea
- Some redundant steel work causes problems

Note: despite extensive and expensive assessment demonstration campaign using every technology you can imagine, no continuous monitoring





Monitoring, black swans & what we don't know



Features of a black swan eventis a surprise (to the observer)has a major impactis rationalized by hindsight

Infrastructure black swan events?

- Tacoma Narrows
- Millennium Bridge
 - 9/11
- Fukushima
- Katrina

A turkey before and after Thanksgiving. The history of a process over a thousand days tells you nothing about what is to happen next. This naïve projection of the future from the past can be applied to anything.

- We don't know what we know
- We don't know what we don't know
- Conclusion?
- We have to learn (share failure stories and remember) from past and be sensitive (use SHM?) to find precursors of doom